# DESIGN OF CENTRALIZED WASTEWATER SEWERAGE SYSTEM IN SMALL LENGKANG ISLAND, BATAM CITY, INDONESIA

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## Abstract

One of the small islands within the Indonesia-Singapore border is Lengkang Island. Wastewater collection infrastructure services on Lengkang Island need to be assessed to improve Sustainable Development Goals (SDGs), where access to wastewater services must reach 100%. The purpose of this study is to determine the existing conditions of wastewater management and to plan a centralized wastewater sewerage system on Lengkang Island involving the community. The existing condition shows that all residents channel wastewater into the sea. Only a few houses have toilets, semi-permanent houses on land. Non-permanent houses built on the sea do not have latrines, where stools are dumped directly into the sea. Tripikon-S is one way to overcome the problem of impermanent homes that do not have latrines. Latrine does not provide a solution to the effects of eutropication. Centralized wastewater collection systems using PVC pipes mentioned the pipe diameter between 32 mm - 60 mm, with a speed of 0.3 m/sec -1.31 m/sec. With this centralized wastewater sewerage system infrastructure, community participation is expected to increase environmentally friendly development.

Kata kunci: sewerage system, small island, wastewater

### Introduction

Riau Islands Province is one of the provinces in Indonesia that gives islands, which means the total sea area is more than the total sea area (Ginting, 2016). One of the cities in the Riau Islands Province is Batam City which consists of Rempang Island, Batam Island, Galang Island, Tonton Island, Nipah Island, and Galang Baru Island and small islands located in the Singapore Strait and the Malacca Strait. Based on Law

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Number 27 of 2007 concerning Management of Coastal Areas and Small Islands, that is, an island which has an area of less than or equal to 2000 km2 and its ecosystem. Environmental pollution due to the disposal of wastewater and feces is often not supported by the provision of environmental sanitation infrastructure (Lalasati and Hadi, 2019). Realizing environmental sanitation infrastructure is supported by the 100-0-100 Movement that answers the challenges of the Sustainable Development Goals (SDGs) in accessing domestic wastewater treatment by 100%. Infrastructure development in the sewerage system is one of the efforts to increase

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the standard of living of the people and decrease the city without slums.

Planning for domestic wastewater management, both latrines and non-latrines, is regulated through Minister of Public Works and Public Housing Regulation No. 04/2017 regarding the Implementation of Domestic Wastewater Management Systems. The regulation does not specify specific planning areas such as small islands like Lengkang Island.

Macrozoobenthos in the waters of Lengkang Island showed that there were 3 phyla, 5 classes, 17 families with 19 species with abundance values between 3317.5 - 3809.5 ind/m<sup>3</sup> (Agustinus et al., 2013). The most feared environmental impacts of improper disposal of wastewater are species dying and biodiversity loss. The death of species can occur due to explosion of algae populations or commonly called eutropication. Eutropication can be caused by nutrient content in wastewater (Suryawan et al., 2019a; Suryawan et al., 2020). Domestic wastewater treatment that can be applied such as constructed wetland and Anaerobic Baffled Reactor (ABR). Contructed wetland can use plants such as Cattail (Hidayah Aditya, 2010), and Cyperus Papyrus (Suprihatin, 2014), and Eichhornia crassipes (Suryawan, 2018). Whereas ABR can utilize bulkhead media as a medium for growing microorganisms and degrades pollutants from wastewater (Suryawan et al., 2019b).

Increasing efforts to channel wastewater is one way to increase community participation through the provision of infrastructure facilities. According to Dupai et al. (2019), this is one factor not a proactive community in development. Ikemi, (2017) states that control and management of community compliance such as water supply, sewage treatment and waste disposal are not provided properly so good planning is needed.

This article was written to find out the existing condition of wastewater management carried out by the community on Lengkang Island. In addition, a centralized wastewater distribution system was designed on Lengkang Island.

## **Design Criteria**

The planned wastewater delivery system is a centralized wastewater delivery system, where all wastewater produced is channeled to a place and treated collectively. This system is used because the onsite system cannot be implemented due to limited land and inadequate land conditions as a result of the high population density sticks on Lengkang Island. The planning data used in this study are:

a. Primary data is a field observation of the number of houses and the current condition of wastewater treatment. In the observation also carried out measurements of the elevation of the field using GPS.

b. Secondary data obtained from the relevant agencies in the form of population data

In planning the wastewater distribution system used the calculation of the discharge which consists of:

 $Qr = Water demand \times population number \times 0.7$ (1)

$$q_{\rm md} = F_{\rm peak} \times Qr \tag{2}$$

$$Qpp = 5 \times P^{0.5} \times qmd \tag{3}$$

where:

- 0.7 = tipical the values required in the Detailed Engineering Planning Guide Service Subsystem and Collection Sub-system
- Qr = average discharge of wastewater (L/sec);
- $F_{peak} = peak-day factor$
- P = total population (thousands of capita);
- q<sub>md</sub> = maximum discharge days unit discharge
   water (L/sec)
- Qpp = peak discharge design of parcel pipe (L/sec)

Hydraulic-Elements Graph for Circular Pipes or Channels





The calculation of the dimensions of the pipeline in planning is based on the Manning formula:

$$Q = \frac{1}{n} R^{2/3} S^{\frac{1}{2}} A \tag{4}$$

Where Q is the discharge, R is the hydraulic radius, S is the slope, and A is the cross-sectional area. The equation for field slope and pipe slope:

$$S = \frac{\Delta H}{L}$$
(5)

In this planning PVC pipes are used as conduits. Where n is for PVC pipe roughness in BOOK A A Detailed Technical Planning Guide for Service Sub-system and Sub-system for Raising the Roughness Coefficient of pipe (n) PVC is 0.002-0.012. To determine the diameter of a pipe using the area equation as in equation 6.

$$A = \frac{1}{4}\pi D^2 \tag{6}$$

The velocity equation when the pipe is full as in equation 7.

$$Vfull = \frac{1}{n} S^{1/2} 0,397 D^{2/3}$$
(7)

The velocity of drainage in the wastewater delivery system must be within certain speed limits. The minimum speed is based on the drainage ability to provide the channel's selfrinsing power against deposits. To prevent organic deposition, a minimum speed of 0.3 m/sec is used. The design of wastewater systems for small scale also uses minimum velocity criteria of 0.3 m/sec (Harindi and Kamil, 2010; Otis and Mara, 1985). The maximum speed is based on the ability of the channel to the possibility of scouring that occurs by flow containing coarse particles. To prevent scouring, the maximum allowable speed is around 3.0 m/sec according to the Detailed Engineering Planning Guide for the Service Sub-system and the Collection Sub-system. The slope in this plan does not have special criteria because it considers sufficient pressure to drain wastewater by gravity.

## **Result and Discussion**

#### Existing Condition

Based on the results of a survey of 146 existing housing units on Little Lengkang Island, only 26

of them were built on land. The rest of the houses inhabited by residents are houses built above the sea or commonly called stilt houses (Figure 2). Almost all houses on Lengkang Island are built in semi-permanent and nonpermanent types. The type of settlement will affect the community's preference for household sanitation facilities, namely the type of house that has a toilet and how to dispose of waste (Lalasati and Hadi, 2019).



Figure 2. The Condition of Non Permanent Houses on Lengkang Island



Figure 3. Examples of Simple Latrine Conditions on Lengkang Island

The results of observations on 107 house samples show black water and gray water are thrown away into the sea (simple pit sewage). As can be seen in Figure 3, the condition of the latrines is carried out directly above the sea. An interview with 107 respondents also stated that

the community did not know the impact that could be caused by inappropriate disposal of wastewater. Another study conducted in the Riau Islands Province, precisely on Penyengat Island, said 27% of the community also used simple pit sewage which was dumped into the sea (Lalasati and Hadi, 2019).

This domestic wastewater usually contains quite high nutrients which can lead to abundant algal growth (Suryawan and Sofiyah, 2020). Eutrophication is a complex process between growth of algal biomass as a whole but also with changes in biodiversity (Glibert, 2017).

At present there are many technologies that are suitable to be applied in the processing of feces in which are suitable for houses on stilts. Tripikon-S is one of the technologies suitable for use in stilt houses in coastal areas (Marlisa et al., 2015; Kusumaningayu, et al., 2015). However, this Tripikon-S technology still has weaknesses where the potential impact of eutropication is  $2.3 \times 10-10$  kgPO4-eq (Noor and Soewondo, 2018). The still high chance of environmental impact on the on-site system causes a centralized processing system still needs to be done.

## Design

The planned wastewater delivery system is a centralized system consisting of line x and line y. The residential area is divided into 7 residential areas. The housing area is housing A, B, C, D, E, F, and G. The design of the wastewater collection system can be seen in Figure 2. The calculation for parcel discharge is carried out by considering the service area for each line. In calculating the Qr, the need for water used is 120 L/person.day. The need is used because the small island of Lengkang is still included in the administrative area of Batam City, based on SNI 19-6728.1-2002 states that the use of water in urban areas is 120 L/person/day. Opp is the total debit discharged by each house. Opp calculation results of each house can be seen in Table. 1.



Figure 4. Map of Centralized Wastewater Sewerage System Planning in Lengkang Island.

nousing								
Number	Qr	$\mathbf{q}_{\mathbf{md}}$	Q <sub>pp</sub>					
of house	L/s	m <sup>3</sup> /s	m <sup>3</sup> /s					
Line X								
21	0.20	0.00018	0.00034					
15	0.15	0.00013	0.00021					
16	0.16	0.00014	0.00023					
Line Y								
15	0.15	0.00013	0.00021					
25	0.25	0.00022	0.00045					
23	0.23	0.00020	0.00040					
31	0.31	0.00027	0.00063					
	Number of house           21           15           16           15           25           23           31	Number of house         Qr           21         0.20           15         0.15           16         0.16           Line         15           21         0.20           15         0.15           26         0.25           23         0.23           31         0.31	Number of house         Qr         qmd           L/s         m³/s           Line X           21         0.20         0.00018           15         0.15         0.00013           16         0.16         0.00014           Line Y         15         0.15         0.00013           25         0.25         0.00022         23         0.23         0.00027           31         0.31         0.00027         0.00027         0.00027					

 Table 1. Qpp calculation results for each

The length of the measured path (L) is obtained based on observations. The value of  $\Delta H$  is determined based on the consideration of the field slope. Field slope tends to decrease from north to south. Discharge calculation results for each path can be seen in Table 2.

To get the value of the diameter of the pipe installed in the channel and check the design criteria for speed the calculation is done.

Table 2. Qpp calculation	results	for	line	and
slope lir	е			

stope line									
Line	L	Qpp line	$\Delta \mathbf{H}$	S					
	m	m³/s	m	m/m					
Line X									
x1-x2	87	0.00034	0.5	0.005747					
x2-x3	83	0.00055	0.5	0.006024					
x3-IPAL	74	0.00078	0.5	0.006757					
Line Y									
y1-y2	101	0.00014	1	0.009901					
y2-y3	84	0.00026	0.5	0.005952					
y3-y4	62	0.00034	0.5	0.008065					
y4-IPAL	25	0.00055	0.5	0.02					

The results of the planning calculations can be seen in Table 3 and Table 4. Examples of calculations are given for lines x1-x2. Data :

Qpp =  $0.00034 \text{ m}^{3}/\text{sec}$   $\Delta \mathbf{H}$  = 0.5 m**S** = 0.005747 m/m

#### Calculation:

The d/D is 0.8, based on the Hydraulic Elements for Circular Sewer graph (Figure 1.), the Qpp / Qfull is 0.975. The calculation steps are as follows:

a. Calculation of Q full (m<sup>3</sup>/s) O full =  $O_{\text{neak}} / (O_{\text{neak}} / O_{\text{full}})$ 

$$= 0.0003 \text{ m}^3/\text{s} / 0.975$$
$$= 0.00035 \text{ m}^3/\text{s}$$

- b. Calculation of theoretical diameter and installed diameter  $0.00035 \text{ m}^3\text{/s} = \frac{0.3117}{0.01} \text{ x } D^{8/3} \text{ x } 0.005747^{\frac{1}{2}}$ D = 0.0363 mD installed= 48 mm
- c. Calculation of Q <sub>full check</sub> (m<sup>3</sup>/s) from theoretical diameter (mm) utilizing equation (4). Q <sub>full</sub> =  $0.00072 \text{ m}^{3}/\text{s}$
- d. Calculating  $Q_{peak}/Q_{full}$  check = 0.00034 m<sup>3</sup>/s / 0.00072 m<sup>3</sup>/s = 0.5
- e. Determine d/D check using Q<sub>peak</sub>/Q<sub>full</sub> check graph, which is 0.5. Using graph in Fig.1, it can be determined that d/D check value is 0.6.
- f. Calculating A<sub>peak</sub> (m<sup>2</sup>)
   Determine wet cross sectional area (Fig. 5)
   in peak condition with following method:

![](_page_6_Figure_1.jpeg)

**Figure 5.** Variables in the Manning equation of partial gravity flow

r	= 0.024  m
$d_{hall}$	$= d/D \times Dinstalled$
	$= 0.6 \times 0.048 \text{ m}$
	= 0.029 m
t	= d - r
	= 0.029  m - 0.024  m
	= 0.0048 m
t/r	= 0.20
α	$= 2 \operatorname{arc} \cos t/r$
	$= 2 \operatorname{arc} \cos 0.20$
	= 156.93°
β	$= 360^{\circ} - 156.93^{\circ}$
	$= 203.07^{\circ}$

$$\begin{split} A_{\text{section}} &= 203.07^{\circ}/360^{\circ} \times \frac{1}{4} \times \pi \times D^{2} \\ &= 203.07^{\circ}/360^{\circ} \times \frac{1}{4} \times \pi \times 0.048^{2} \\ &= 0.001 \text{ m}^{2} \\ A_{\text{triangle}} &= \frac{1}{2} \times a \times t \\ &= \frac{1}{2} \times 2 \times (0.024^{2} - 0.0048^{2})^{0.5} \times 0.22 \\ &= 0.00011 \text{ m}^{2} \\ A_{\text{PEAK}} &= A_{\text{section}} + A_{\text{triangle}} \\ &= 0.001 + 0.00011 \\ &= 0.0011 \text{ m}^{2} \\ \end{split}$$

$$\begin{aligned} \text{Calculating V}_{\text{peak}} (\text{m/s}) \\ \text{v}_{\text{peak}} &= Q_{\text{pp}} / A_{\text{peak}} \end{aligned}$$

 $\begin{array}{ll} g. & Calculating \, V_{peak} \, \left( m/s \right) \\ v_{peak} & = Q_{pp}/\, A_{peak} \\ & = 0.00034/\, 0{,}0011 \\ & = 0.30 \; m/s \; (suitable) \end{array}$ 

$$\begin{array}{ll} h. & Calculating \, V_{full} \, \left( m/s \right) \\ V_{full} & = v_{peak}/( \, Q_{pp}/Q_{full}) \\ & = 0.30 \ / \ 0,5 \\ & = 0.63 \ m/s \ (suitable) \end{array}$$

Table 3. The results of the diameter calculation in the centralized wastewater sewerage system

Line	Qpp 24	d/D rencana	Q pp/ Q full	Q full n		D n <u>calculated i</u> n		Q full chek	Qpeak/ - Qfull chek	d/D chek
	m³/s		rencana	m³/s		m Line V	mm	m³/s	-	
						Line A				
x1-x2	0.0003	0.80	0.975	0.00035	0.01	0.0363	48	0.00072	0.5	0.60
x2-x3	0.0005	0.80	0.975	0.00056	0.01	0.0430	48	0.00074	0.7	0.81
x3-IPAL	0.0008	0.80	0.975	0.00080	0.01	0.0479	60	0.00141	0.6	0.68
Line Y										
y1-y2	0.0001	0.80	0.975	0.00014	0.01	0.0232	32	0.00032	0.4	0.55
y2-y3	0.0003	0.80	0.975	0.00027	0.01	0.0328	42	0.00051	0.5	0.60
y3-y4	0.0003	0.80	0.975	0.00035	0.01	0.0342	48	0.00085	0.4	0.55
y4-IPAL	0.0005	0.80	0.975	0.00056	0.01	0.0343	48	0.00134	0.4	0.55

Line	r	d hall	t	t/r	α	β	A juring	A triangle	Apeak	Vpeak	Vfull
	m	m	m	m/m	0	0	<b>m</b> <sup>2</sup>	<b>m</b> <sup>2</sup>	m <sup>2</sup>	m/sec	m/sec
Line X											
x1-x2	0.0240	0.0288	0.0048	0.20	156.93	203.07	0.00102	0.00011	0.00113	0.30	0.63
x2-x3	0.0240	0.0389	0.0149	0.62	103.37	256.63	0.00129	0.00028	0.00157	0.35	0.47
x3-IPAL	0.0300	0.0408	0.0108	0.36	137.80	222.20	0.00174	0.00030	0.00205	0.38	0.69
					Li	ne Y					
y1-y2	0.0160	0.0176	0.0016	0.10	168.52	191.48	0.00043	0.00003	0.00045	0.30	0.71
y2-y3	0.0210	0.0252	0.0042	0.20	156.93	203.07	0.00078	0.00009	0.00087	0.31	0.59
y3-y4	0.0240	0.0264	0.0024	0.10	168.52	191.48	0.00096	0.00006	0.00102	0.34	0.83
y4-IPAL	0.0240	0.0264	0.0024	0.10	168.52	191.48	0.00096	0.00006	0.00102	0.54	1.31

**Table 4.** The results of the calculation of speed in a centralized wastewater sewerag system

The results of Dupai et al., (2019) research on factors the Kendari coastal community influencing environmental awareness behavior are knowledge, attitudes and behaviors while the determinants that shape clean and healthy living behaviors infrastructure availability, are stakeholder intervention, routine monitoring by stakeholders, the threat of health risks due to unhealthy behavior, and the habits of coastal communities so as to enhance the environmental health behaviors of coastal communities. Thus, with the existence of a centralized wastewater distribution system infrastructure, it is expected that the living standards of the Lengkang Island coastal community will be met.

## Conclusion

The existing condition of domestic wastewater management on Lengkang Island uses a simple pit sewage system which is directly channeled to the sea. This condition will cause environmental impacts both biotic and abiotic. The planning of a centralized wastewater sewerage system requires a suitable technical study that uses a gravity system with a pipe diameter of 32 mm-60 3 mm, with the velocity of each path of 0.3-1.31 m/sec.

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