THE USE OF MULTI-CRITERIA ANALYSIS IN SELECTING WATER TREATMENT UNITS IN SADU WATER TREATMENT PLANT, BANDUNG DISTRICT, WEST JAVA PROVINCE, INDONESIA

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Abstract

Increasing the number of residents and still not fulfilling the scope of drinking water services for resident of Bandung Regency, Tirta Raharja Regional Water Supply Company is developing in the field of drinking water supply. To improve community drinking water services in the Bandung Regency area, Tirta Raharja Water Supply Company plans to add a drinking water treatment plant with a capacity of 150 liters/second located in Sadu Village, Soreang District, Bandung Regency. This research aims to use of multi-criteria analysis in selecting water treatment units in Sadu WTP. In the planning of the Sadu WTP (Sadu Water Treatment Plant) was carried out several stages of planning, i.e. literature study, preliminary survey, data collection, alternative selection of treatment units using multi criteria analysis, and calculation of treatment unit dimensions. The parameters under review are those that exceed the standard of quality according to the Minister of Health Regulation No. 492 of 2010, i.e. turbidity, BOD, COD and fecal coliform. The selection of Sadu WTP units is based on parameters that exceed drinking water quality standards. There were two alternative proposed (Alternative I and Alternative II) in drinking water treatment, i.e. complete unit (Alternative I) consisting of intake; hydraulic coagulation; hydraulic flocculation; tube settler sedimentation; rapid sand filter; reservoir; disinfection; and sludge drying bed, and slow sand filter unit (Alternative II) consisting of intake, pre-sedimentation, slow sand filter, reservoir, disinfection, and sludge drying bed. The method to be used in determining the unit plan is multi-criteria analysis which refers to aspects based on Minister of Public Works Regulation No. 18/PRT/M/2007. Based on the results of the scoring obtained, the complete treatment unit has the highest score, i.e. 4.05, so the alternative treatment used in the Sadu WTP is the complete treatment unit (Alternative I).

Keywords: alternative water treatment, multi criteria analysis, water treatment plant, water quality

Introduction

Drinking water is one of the sanitation needs of humankind (Hasbiah et.al, 2019). The demand for water worldwide has increased substantially over the past decades. Development analysts attribute it to the growing population numbers, increased irrigation areas, economic development and acute water shortage in many parts of the world. This development has resulted in the drying up of rivers. The issues concerning water usage are sectoral and affect both inputs and outputs of the national and international economics. The agricultural sector, production of energy, industrial uses, and human consumption are the main areas where the demand for water is high. Among the sectors

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enumerated, livestock and the crops production sector are the intensive-users of water (Udimal et.al, 2017).

Water treatment plants (WTPs) are characterized as complex configurations of repairable and deteriorating components (Smith et.al, 2019). Water suppliers are a variety of treatment processes to remove contaminants from raw water. These individual processes may be arranged in a "treatment train" (a series of processes applied in sequence). The most commonly used processes include filtration, flocculation and sedimentation and disinfection for surface water. Some treatment trains also include ion exchange and adsorption. Water utilities select a combination of treatment processes that is the most appropriate to treat the contaminants found in the raw water (Angreni, 2009).

The restriction of water's resources, the need of a careful management and the importance to ensure a good water quality are now more obvious than ever. Treatment of water is accomplished through process of mechanical nature (retention on grates and separators, sediment exclusion, decantation, filtration), of chemical nature (coagulation-flocculation, ion exchange, chlorination, disinfection with UV radiations, aeration-oxidation) and of biological nature (adsorption on active coal, biological treatment using slow filter or semi – permeable membrane) (Oana et.al, 2011).

Increasing the number of residents and still not fulfilling the scope of drinking water services for resident of Bandung Regency, Tirta Raharja Regional Water Supply Company is developing in the field of drinking water supply. Tirta Raharja Water Supply Company currently serves three administrative regions, i.e. Bandung Regency, Cimahi City and West Bandung Regency. In Bandung Regency, there are six branches or service units. The total production capacity of Tirta Raharja Water Supply Company in Bandung Regency until 2018 is 885 liters/second with six Water Treatment Plants (WTPs) divided into three service areas, i.e. the south, north and east. Tirta Raharja Water Supply Company until 2018 has had 97,329 subscription connections. To improve community drinking water services in the Bandung Regency area, Tirta Raharja Water Supply Company plans to add a drinking water tratment plant with a capacity of 150 liters/second located in Sadu Village, Soreang District, Bandung Regency.

Analysis Method

In the planning of the Sadu WTP (Sadu Water Treatmnet Plant) was carried out several stages of planning, i.e. literature study, preliminary survey, data collection, alternative selection of treatment units using multi criteria analysis, and calculation of treatment unit dimensions.

Preliminary Survey

The preliminary survey was intended to provide a direct observation of the actual location of the planning area.

Data Collection

Sampling of raw water was taken from the location of the intake plan using the Grab Sampling method. Samples were taken three times during rainy or wet seasons and three times in dry seasons. The raw water sampling was carried out in accordance with SNI 7828: 2012 with the sampling done at a depth of approximately half the river depth.

Multi Criteria Analysis

An alternative analysis of the treatment unit was conducted to select an effective and efficient treatment unit based on the Cisondari River water quality that exceeds the standard according to Minister of Health Regulation No. 492 of 2010. The method to be used in determining the unit plan is multi-criteria analysis which refers to aspects based on Minister of Public Works Regulation No. 18/PRT/M/2007.

Calculation of Treatment Units

Calculation of selected unit dimensions was conducted using the design criteria obtained from the existing WTP evaluation, namely the Cipageran WTP. Evaluations for Sadu WTP can be calculated by the equation below (Qasim, 2000):

Velocity	$v = \frac{Q}{2}$	(1)
	Δ	

Detention time
$$t = -\frac{v}{2}$$
 (2)

Gradien
$$G = \sqrt{\frac{P}{\mu \times V}}$$
(3)

Hydraulic Gradien	$Gt_d = G \ x \ t_d$	(4)
Reynolds number	$N_{Re} = \frac{V p x R}{v}$	(5)
Froude number	$N_{\rm Fr} = \frac{V p^2}{g x R}$	(6)

Where Q is discharge of raw water (m³/second), V is tank volume (m³), μ is absolute viscocity (kg/m.second), and ρ is density (kg/m³).

Result and Discussion

Raw Water Quality Analysis In Sadu WTP plan, the source of raw water used is sourced from the upper reaches of the Cisondari River with a planned capacity of 150 liters/second. The selection of the Cisondari River as a source of raw water is due to the Cisondari River's minimum discharge still meets the capacity of the plan if tapping is carried out and the quality of the Cisondari River still meets the requirements as a source of drinking water.

Sampling of raw water was done in two seasons, i.e. dry and wet seasons. Sampling of raw water that represents the wet season was conducted on January 28, 2019, while sampling of raw water representing the dry season was conducted on March 12, 2019. The results of the analysis of the raw water quality of the Cisondari River were then compared with Government Regulation No. 82 of 2011 Class I concerning Water Quality Management to see the feasibility of Cisondari River water as raw water that will be processed into drinking water. The results of the analysis were also compared with Minister of Health Regulation No. 492 of 2010 concerning Drinking Water Quality Requirements to determine parameters that exceed drinking water standards. The results of the analysis of raw water quality is shown in Table 1.

Table 1. Comparison of Raw Water Quality Results with Government Regulation No. 82 of 2001 and Minister
of Health Regulation No. 492 of 2010

		Government Minister of		Minister of	Results			
No.	Parameters Units Regulation No		Health Regulation No. 492 of 2010	March 12, 2019	January 28, 2019			
A. P	hysics							
1	Temperature	°C	Deviation 3	Air temperature ±3°C	24	26		
2	Turbidity	mg/l	-	5	7.5	28.3		
3	Total Dissolved Solid (TDS)	mg/l	1000	500	14	17		
4	Total Suspended Solid (TSS)	mg/l	50	-	54	7.7		
Cher	mistry							
1	pH	-	-	6.5 - 8.5	7.2	7.2		
2	BOD ₅	mg/l	2	2	0.7	11		
3	COD	mg/l	10	10	1.59	35.4		
4	Dissolved Oxygen (DO)	mg/l	6	-	4.63	7.4		

			Government	Minister of	Results			
No.	Parameters	Units	Regulation No. 82 of 2011	Health Regulation No. 492 of 2010	March 12, 2019	January 28, 2019		
5	Total phosphate (PO ₄ -P)	mg/l	0.2	-	0.65	0.02		
6	Nitrate (NO ₃ -N)	mg/l	10	50	0.67	0.8		
7	Nitrite (NO ₂)	mg/l	-	3	< 0.01	< 0.026		
8	Free ammonia (NH ₃ -N)	mg/l	0.5	1.5	< 0.05	< 0.0618		
9	Arsen (As)	mg/l	0.05	-	< 0.002	< 0.002		
10	Cobalt (Co)	mg/l	0.2	-	< 0.04	< 0.076		
11	Barium (Ba)	mg/l	1	-	< 0.12	< 0.0419		
12	Selenium (Se)	mg/l	0.01	0.01	< 0.001	< 0.001		
13	Cadmium (Cd)	mg/l	0.01	-	< 0.003	< 0.0016		
14	Chromium hexavalent (Cr(VI))	mg/l	0.05	-	< 0.02	< 0.005		
15	Copper (Cu)	mg/l	0.02	2	< 0.02	< 0.0083		
16	Iron (Fe)	mg/l	0.3	0.3	< 0.04	< 0.01		
17	Lead (Pb)	mg/l	0.03	-	< 0.02	< 0.022		
18	Manganese (Mn)	mg/l	0.1	0.4	< 0.02	< 0.1		
19	Mercury (Hg)	mg/l	0.001	-	< 0.0008	< 0.0005		
20	Zinc (Zn)	mg/l	0.05	3	< 0.01	< 0.005		
21	Chloride (Cl)	mg/l	600	250	<1.9	35.4		
22	Cyanide (CN)	mg/l	0.02	0.07	< 0.01	< 0.005		
23	Fluoride (F)	mg/l	0.5	-	< 0.05	< 0.92		
C. M	licrobiology							
1	Fecal coliform	cell/100 ml	100	-	0	<3		
2	E. Coli	cell/100 ml	-	0	0	3		
3	Total coliform	cell/100 ml	1000	0	36	3		

Remarks:

Parameter that exceed quality standards_

As illustrated in Table 1, it is shown that there were parameters that exceed the standards, i.e. turbidity, total suspended solid (TSS), biological oxygen demand (BOD), chemical oxygen demand (COD), dissolved oxygen (DO), total phosphate (PO₄-P), E.coli, and total coliform. Therefore, water treatment is needed so that raw water sources can be used as drinking water. However in designing drinking water treatment, the parameters under review are those that exceed the standard of quality according to the Minister of Health Regulation No. 492 of 2010, i.e. turbidity, BOD, COD and fecal coliform.

Drinking Water Treatment Alternatives

The selection of Sadu WTP units is based on parameters that exceed drinking water quality standards. Coagulation, flocculation, and sedimentation units are used to remove turbidity, TSS, BOD, COD, total phosphate, fecal coliform and total coliform parameters. The filtration unit can remove turbidity, fecal coliform, E.coli, and total coliform parameters. Alternative water treatment plants in Sadu WTP can be seen in Table 2.

-	Table 2. Alternative water freatment Frants in Sadu wiff																	
	Drinking Water Quality					Pre	treatm	ents			Treatn	nents		S	pecial	Treatm	ents	
No	Parameters	Dry Season Measurement	Wet Season Measurement	Quality Standard *)	Units	S	PC	PS	A	LS	CS	RSF	SSF	Р	SC	AC	SCT	SWT
1	Turbidity	7.5	28.3	5	NTU							\checkmark	\checkmark					
2	BOD	0.7	11	2	mg/l								\checkmark					
3	COD	1.59	35.4	10	mg/l								\checkmark					
4	Fecal coliform	0	3	0	MPN/ 100 ml						\checkmark	\checkmark	\checkmark	\checkmark				
5	Total coliform	36	3	0	MPN/ 100 ml						\checkmark	\checkmark	\checkmark	\checkmark				

Table 2. Alternative Water Treatment Plants in Sadu WTP

Remarks:

 $\sqrt{}$ = Unit used

- *) = Minister of Health Regulation No. 492 of 2010
- S = Screening
- PC = Pre Chlorination
- PS = Pre-sedimentation
- A = Aeration
- LS = Lime Softening
- CS = Coagulation flocculation & sedimentation

A. Drinking Water Treatment Alternative I

Based on the raw water parameter data that needs to be removed, Alternative I combination of operating and processing units that more specific are as follows:

- 1. Intake;
- 2. Hydraulic coagulation;
- 3. Hydraulic flocculation;
- 4. Tube settler 60° sedimentation;
- 5. Rapid sand filter;
- 6. Reservoir;
- 7. Disinfection using chlorine chemicals in the reservoir;
- 8. Sludge treatment using sludge drying bed.



Figure 2. Scheme of Drinking Water Treatment Alternative I.

B. Drinking Water Treatment Alternative II Based on the raw water parameter data that needs to be removed, Alternative II combination

RSF	= Rapid	Sand	Filter
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SSF = Slow Sand Filter

- P = Post Chlorination
- SC = Super Chlorination
- AC = Activated Carbon
- SCT = Special Chemical Treatment
- SWT = Salt Water Treatment

of operating and processing units that more specific are as follows:

- 1. Intake;
- 2. Pre-sedimentation;
- 3. Slow sand filter:
- 4. Reservoir:
- 5. Disinfection using chlorine chemicals in the reservoir;
- 6. Sludge treatment using sludge drying bed.



Figure 3. Scheme of Drinking Water Treatment Alternative II.

Multi-criteria Analysis in Selecting Water Treatment Units

There were two alternative proposed (Alternative I and Alternative II) in drinking water treatment, i.e. complete unit (Alternative I) consisting of intake; hydraulic coagulation; hydraulic flocculation; tube settler sedimentation; rapid sand filter; reservoir; disinfection; and sludge drying bed, and slow sand filter unit (Alternative II) consisting of intake, pre-sedimentation, slow sand filter, reservoir, disinfection, and sludge drying bed. Based on alternative unit choices available, an appropriate method is needed to determine the treatment units that will be used in the WTP plan. The method to be used in determining the unit plan is multi-criteria analysis which refers to aspects based on Minister of Public Works Regulation No. 18/PRT/M/2007. Each aspect is given a different weight. The total weight for all aspects is 100%. The weighting classification is shown in Table 3.

No.	Criteria	Weight Percentage	ge Description/Reasons for Weighting				
Tech	nical Aspects	<u> </u>					
1	Removal efficiency in treatment process	20%	This aspect is influential due to the higher the treatment efficiency, the better the quality produced.				
2	The flexibility of the treatment system to quality fluctuations	10%	This aspect is not very influential due to the treated water discharge has been adjusted to the provisions of the planned discharge, and the quality of the Cisondari River is not fluctuate too much due to it is located upstream of the river.				
3	Construction	5%	This aspect is not very influential due to construction seen from the level of simplicity. The easier the construction of a treatment plant, the better it will be.				
4	Operational and maintenance	15%	This aspect has an effect on the operation in accordance with the SOP for the workings of the WTP.				
5	Availability of WTP material units	5%	This aspect is quite influential due to the material needed affects the construction to be constructed.				
6	Possible damage	5%	This aspect is not very influential because if a unit plan is very concerned about operational and maintenance factors of the unit, the possibility of damage is small.				
Ecor	omic Aspects						
7	The land area required	10%	This aspect is quite influential due to alternative WTP units will be made by requiring a large enough land for planning.				
8	Unit operating and maintenance costs	15%	This aspect needs to be considered, due to these costs will be incurred while the unit is operating. These costs include energy costs incurred, costs for chemicals used, the cost of residue treatment, and salaries of employees/labors.				
9	Construction costs	10%	This aspect is considered due to construction costs are costs incurred at the beginning of construction.				
Envi	ronmental Aspects						
10	Ecological balance, environmental carrying capacity and land use	5%	This aspect is not too much of a consideration in choosing a drinking water treatment, due to every residue produced will be reprocessed so that it does not pollute the environment.				

Table 3. Classification of Weighting Multi-Criteria Aspects

Weights in this aspect will be used as an assessment of the available alternative units by comparing the strengths and weaknesses of each alternative choice. The assessment is conducted by giving a score of 1 to 5, the higher the score obtained for each unit, the better the

performance. The total value for each aspect is calculated by multiplying the weight in percent by the given score. The results of calculating alternative score of complete unit and slow sand filter are shown in Table 4.

Criteria	Weight (%)	-	reatment Unit native I)	Slow Sand Filter (Alternative II)		
		Value	Score	Value	Score	
Fechnical	Aspects					
1	20%	5	1	3	0.6	
2	10%	5	0.5	3	0.3	
3	5%	4	0.2	5	0.25	
4	15%	3	0.45	4	0.6	
5	5%	3	0.15	4	0.2	
6	5%	5	0.25	4	0.2	
Economic	Aspects					
7	10%	4	0.4	5	0.5	
8	15%	4	0.6	5	0.75	
9	10%	3	0.3	4	0.4	
Environm	ental Aspects					
10	5%	4	0.2	3	0.15	
Total	100%		4.05		3.95	

Table 4. Comparison of Alternative I and Alternative II Scores

Based on the results of the scoring obtained, the complete treatment unit has the highest score, so the alternative treatment used in the Sadu WTP is the complete treatment unit (Alternative I). After determining the alternative treatment selected, then it can be determined the type of drinking water treatment to be used. Drinking water treatment unit recommendation can be seen in Table 5.

No.	Selected Unit	Recommendation
		The intake that will used in the planning of Sadu WTP is river intake. Water from
1	Intake	the Cisondari River is flowed to the collection basin using a carrier channel. Raw
		water flows to the location of the treatment unit by gravity.
		In Sadu WTP, the type of coagulation that will be used is hydraulic coagulation,
2	Coagulation	i.e. by utilizing the waterfall. The coagulant to be used is aluminum sulfate at a
		dose of 20 mg/l.
		Flocculation that will be used in the planning of Sadu WTP is hydraulic
		flocculation with a baffled channel system. The consideration of the type of
3	Flocculation	hydraulic unit is based on the type of unit used at the equivalent WTP (Cipageran
		WTP) and the efficiency of the costs to be incurred. Flocculation tanks will be
		made as much as 1 unit with 6 compartments.
4	Sedimentation	In the planning of the WTP, two sedimentation units will be built with vertical flow
4	Sedimentation	and use tube settlers with a slope of 60° in the settling zone.
		In the planned WTP units, 5 filtration tanks will be made. The type of filtration
5	Filtration	used is the rapid sand filter by using two media, i.e. sand and anthrasite and buffer
5	Fillation	media, i.e. gravel. The use of dual media was chosen so that it can filter water with
		higher turbidity and better operating time for the unit.
6	Reservoir	In the planned WTP, a reservoir unit will be built with a capacity of 270 m ³ each.

Table 5. Sadu WTP Alternative Treatment Unit Selected

Design of Sadu Water Treatment Plant A. Intake

The intake unit is planned to use river intake, the flow of raw water from the river to the WTP flowing gravity. The river intakes, dependent on the formation of the river bed and the amount of total suspended solids of raw water could be located onshore with the intake line preferably on piles to convey water into the onshore sump for pumping or it could be a simple offshore jetty structure to support low head borehole pumps or high pressure centrifugal pumps as required by the system (Baharodi, 2016). River intake is a type which may either located sufficiently inside the river so that demands of water are met with in all the seasons of the year, or it may be located near the river bank where a sufficient depth of water is available (Anupojo, 2016). The intake unit consists of sluice gate, bar screen, carrier channel and collecting well. The distance of the intake to the WTP is as far as 16.5 km. Design of intake can be seen in Table 6 and intake unit sketch can be seen in Figure 4.

Table 6. Design of Intake in Sadu WTP

Description	Symbol	Unit	Obtained Value							
Sluice gate										
Sluice gate width		m	0.525							
The height of the sluice gate opening		m	2.11							
er channel										
Carrier channel length		m	6							
Carrier channel width		m	0.525							
Carrier channel height		m	2.11							
Average flow velocity of the carrier channel		m/sec	0.67							
cting well										
Collecting well length		m	3							
Collecting well width		m	1.5							
Collecting well height		m	3.61							
ure release tank and transmiss	ion pipe									
Number of press release tanks		tanks	4							
Transmission pipe diameter		mm	250							
Flow velocity	v	m/sec	3							
Press release tank length		m	2.6							
Press release tank width		m	1.3							
Press release tank height		m	2							
Detention time	t _d	second	45							
	e gate Sluice gate width The height of the sluice gate opening er channel Carrier channel length Carrier channel width Carrier channel width Carrier channel height Average flow velocity of the carrier channel cting well Collecting well length Collecting well width Collecting well height ure release tank and transmiss Number of press release tanks Transmission pipe diameter Flow velocity Press release tank width Press release tank height	e gate Sluice gate width The height of the sluice gate opening er channel Carrier channel length Carrier channel width Carrier channel height Average flow velocity of the carrier channel cting well Collecting well length Collecting well width Collecting well height ure release tank and transmission pipe Number of press release tanks Transmission pipe diameter Flow velocity v Press release tank length Press release tank width Press release tank height	e gate m Sluice gate width m The height of the sluice gate m opening m er channel m Carrier channel length m Carrier channel width m Carrier channel height m Average flow velocity of m/sec the carrier channel m/sec cting well m Collecting well length m Collecting well height m ure release tank and transmission pipe m Number of press release tanks tanks Transmission pipe diameter mm Flow velocity v m/sec Press release tank length m m Press release tank width m m							



Figure 4. Intake Unit Sketch.

B. Bar screen

Screens are used in water treatment for the removal of coarse solids. Screens are either manually or mechanical cleaned. The type of screen used in Sadu WTP is a manual screen. Design of bar screen and bar screen sketch in Sadu WTP can be seen in Table 7 and Figure 5 respectively.

Table 7. Design of Bar screen in Sadu WTP

No.	Description	Symbol	Unit	Obtained Value
1	Stem width	W	m	0.015
2	Opening width	b	m	0.075
3	Total opening width		m	0.525
4	Slope		0	90
5	Number of stems	n	bar	5
6	Number of openings		gaps	6



Figure 5. Bar screen Sketch

C. Coagulation

A very important step in water and in wastewater treatment is the coagulationflocculation process, which is widely used, due cost-effectiveness. its simplicity and to Regardless of the nature of the treated sample (e.g. various types of water or wastewater) and the overall applied treatment scheme, coagulation-flocculation is usually included, either as pre-, or as post-treatment step. The whole coagulationtreatment process flocculation can be divided into two distinct procedures, which should be applied consecutively. The first one termed coagulation, is the process whereby destabilization of a given colloidal suspension or solution is taking place. The function of coagulation is to overcome the factors that promote the stability of a given system. It is achieved with the use of appropriate chemicals, usually aluminum or iron salts, the so-called coagulant agents. Coagulation usually completes in a very short period time of time (e.g. about 10 seconds) (Tzoupanos et.al, 2008).

Coagulation unit of Sadu WTP uses a type of hydraulic mixing. In the coagulation process, there is one receiving tub to collect raw water before entering the stirring tank and given the coagulant material. The raw water inlet pipe in the receiving bath is 250 mm. The coagulant material used in Sadu WTP is aluminum sulfate with dose of 20 mg/l. Design of coagulation tank and coagulation unit sketch can be seen in Table 8 and Figure 6 respectively.

Table 8. Design of Coagulation Tank in SaduWTP

No.	Description	Symbol	Unit	Obtained Value
Rece	iving tank			
1	Number of units		tank	1
2	Detention time	t _d	second	30.2
3	Length of tank	L	m	2.6
4	Tank width	W	m	1.3
5	Tank height	Н	m	1.8
6	Freeboard		m	0.46
Coag	ulation tank			
7	Detention time	t _d	second	9,05
8	Velocity gradient	G	second ⁻¹	745.48
9	Length of tank	L	m	1.5
10	Tank width	W	m	1.3
11	Tank height	Н	m	1.623
12	The height of the waterfall	h	m	0.44
13	Mixing power	Р	Watt	801.75



Figure 6. Coagulation Unit Sketch.

D. Flocculation

The second sub-process after coagulation is flocculation, refers to the induction of destabilized particles in order to come together, to make contact and thereby, to form large agglomerates, which can be separated easier usually through gravity settling. Flocculation occurs usually over a period of 20 to 45 minutes. Coagulation-flocculation is used widely during water or wastewater treatment. It is an integral treatment step in the surface or underground treatment. indented for human waters consumption. Typical applications are the removal/separation of colloids and suspended particles, of natural organic matter, or of metal ions (Tzoupanos et.al, 2008).

Flocculation unit of Sadu WTP uses a type of hydraulic flocculation with vertical flow and consists of 6 hexagonal compartments. The velocity gradient of each compartment is different, starting with the highest compartment 1, followed by the decreasing velocity gradient of the other compartments. Design of flocculation tank and flocculation tank sketch can be seen in Table 9 and Figure 4 respectively.

Table 9. Design of Flocculation Tank in Sadu WTP

No.	Description	Symbol	Unit	Obtained Value
1	Total detention time	t _d total	second	773.04
2	Detention time per compartment	td	second	128.84
3	Hexagonal side		m	1.2
4	Tank height	Н	m	5.5
5	Height of sluice gate opening		m	0.85
6	Width of sluice gate opening		m	0.9
7	Gt _d total	Gt _d		48,368.5
Com	partment 1			
8	Velocity gradient	G_1	second-1	76.89
9	Gt _d	Gt _{d1}		9906.5
10	Flow velocity	\mathbf{v}_1	m/sec	2.11
Com	partment 2			
11	Velocity gradient	G_2	second-1	71.39
12	Gt _d	Gt _{d2}		9137
13	Flow velocity	v ₂	m/sec	1.96
Com	partment 3			
14	Velocity gradient	G ₃	second-1	65.76
15	Gt _d	Gt _{d3}		8375
16	Flow velocity	V ₃	m/sec	1.8
Com	partment 4			

No.	Description	Symbol	Unit	Obtained Value
17	Velocity gradient	G_4	second ⁻¹	62.76
18	Gt _d	Gt _{d4}		7957
19	Flow velocity	V 4	m/sec	1.72
Com	partment 5			
20	Velocity gradient	G ₅	second-1	59.43
21	Gt _d	Gt _{d5}		7504
22	Flow velocity	V5	m/sec	1.63
Com	partment 6			
23	Velocity gradient	G_6	second-1	41.32
24	Gt _d	Gt _{d6}		5489
25	Flow velocity	V6	m/sec	0.25



Figure 7. Flocculation Unit Layout.



Figure 8. Sketch of Flocculation Unit Water Level Reduction.

E. Sedimentation

Sedimentation is a physical water treatment process using gravity to removes suspended solids from water. Solid particles entrained by the turbulence of moving water may be removed naturally by sedimentation in the still water of lakes and oceans. Settling basin are ponds constructed for the purpose of removing entrained solids by sedimentation. Conventional settling units represent around one-third of the total capital cost of water treatment plants because of land and construction costs. Many alternatives have been developed to enhance the performance of settling tanks, increase their hydraulic capacity and decrease their capital or operational costs. Some of these alternatives, such as inclined plates, increase the surface area available for settling and enhance the hydraulic conditions, whereas others, such as alum, improve the settling properties of the suspension (Al-Kizwini, 2015).

Sedimentation unit of Sadu WTP is divided into two tanks. Each sedimentation tank is equipped with a tube settler with a slope of 60° and there are two sludge spaces per sedimentation tank. In the sedimentation basin, the process of settling or separating particles contained in water is deposited by gravity. Design of sedimentation tank and sedimentation tank sketch can be seen in Table 10 and Figure 9.

Table 10. Design of Sedimentation Tank inSadu WTP

No.	Description	Symbol	Unit	Obtained Value
Dime	nsions of Sedimenta	tion Tank		
1	Number of tanks		amounts	2
2	Discharge per tank	Q	m ³ /second	0.075
3	Surface loading	V ₀	m ³ /m ² .sec	1.076 × 10 ⁻³
4	Length of tank	L	m	17.8
5	Tank width	W	m	4
6	Tank height	Η	m	5.1
Settli	ng zone			
7	Tube settler slope		0	60
8	Thick tube		mm	0.7
9	Length of tube		m	1
10	Tube height		m	1
11	Number of tubes		tubes	9398



Figure 9. Sedimentation Unit Sketch

F. Filtration

The main process of water treatment can be mechanical, chemical and biological. From all mechanical treatment procedures, filtration is the most irreplaceable one in the scheme of a treatment plant. Filtration is the advanced clearing procedure, consisting of water passing through a porous material that has a certain granulometry, named filter layer, used for the retention of the natural suspended particles or previous coagulated particles. Filtering is influenced by a series of parameters (Oana, 2011).

The filtration unit is designed using a rapid sand filter type with dual media filters, i.e. sand and anthrasite media and added gravel buffer. The filtration unit consists of 5 tanks with a capacity of 30 l/sec per tank. Design of filtration tank and filtration tank sketch in Sadu WTP can be seen in Table 11 and Figure 10 respectively.

No.	Description	Symbol	Unit	Obtained Value
1	Discharge of filtration	Qtotal	m ³ /sec	0.15
2	Number of tank		tanks	5
3	Discharge per tank	Q	m ³ /sec	0.03
4	Length of tank	L	m	7.2
5	Tank width	W	m	2.4
6	Filtration velocity	v	m/sec	6.25
7	Anthrasite media thickness		cm	40
8	Sand media thickness		cm	25
9	Gravel media thickness		cm	20

Table 11. Design of Filtration Tank in Sadu
WTP



Figure 10. Filtration Unit Sketch

G. Reservoir

The function of the reservoir is to hold water that will be distributed to consumers (Alfatih, et.al, 2017), (Setyowati, et.al, 2016). Filtered water can be used for drinking water. Reservoir is water storage tank located at the installation. The treated water is stored in this tank for later transfer to the distribution system. Design of reservoir includes selection of size and shape; other considerations include protection against stored water, protection of reservoir structure, and protection of reservoir maintenance workers (Qasim, 2000).

Chlorine is added to the water to kill and/or inactive any remaining pathogens. Water is often

disinfected before it enters the distribution system to ensure that potentially dangerous microbes are killed. Chlorine, chloramines, or chlorine dioxide are most often used because they are very effective disinfectants, not only at the treatment plant but also in the pipes that distribute water to our homes and businesses (Angreni, 2009).

In Sadu WTP, the chlorination process takes place after the treatment (post chlorination), i.e. by adding chlorine to the reservoir. The concentration of chlorine used in Sadu WTP is 12% with a chlorine dose of 2 mg/l. Chlorine needs per month is 777.6 kg/month. Design of reservoir tank and reservoir tank sketch can be seen in Table 11 and Figure 11 respectively.

Table 11. Design of Reservoir in Sadu WTP

No.	Description	Symbol	Unit	Obtained Value
Disin	fection			
1	Chlorine		%	12
1	concentration		/0	12
2	Chlorine dose		mg/l	2
3	Chlorine volume		m ³	0.022
4	Number of chlorine		tanks	2
4	reactor tanks		taliks	2
5	Tank height	Н	m	0.5
6	Freeboard		m	0.3
7	Tank diameter	D	m	1.4
8	Diameter of	d	mm	10
0	chlorine pipe			
Reser	voir			
9	Detention time	t _d	minutes	30
10	Number of tanks		tank	1
11	Length of tank	L	m	10.4
12	Tank width	W	m	5.2
13	Tank height	Н	m	5.5



Figure 11. Reservoir Unit Sketch.

Conclusion

In designing drinking water treatment, the parameters under review are those that exceed the standard of quality according to the Minister of Health Regulation No. 492 of 2010, i.e. turbidity, BOD, COD and fecal coliform. There were two alternative proposed (Alternative I and Alternative II) in drinking water treatment, i.e. complete unit (Alternative I) consisting of hydraulic intake; coagulation; hydraulic flocculation; tube settler sedimentation; rapid sand filter; reservoir; disinfection; and sludge drying bed, and slow sand filter unit (Alternative II) consisting of intake, pre-sedimentation, slow sand filter, reservoir, disinfection, and sludge drying bed. Based on the results of the scoring obtained in multi criteria analysis, the complete treatment unit has the highest score, i.e. 4.05, so the alternative treatment used in the Sadu WTP is the complete treatment unit (Alternative I). The intake unit is planned to use river intake, the flow of raw water from the river to the WTP flowing gravity. Coagulation unit of Sadu WTP uses a type of hydraulic mixing. Flocculation unit of Sadu WTP uses a type of hydraulic flocculation with vertical flow and consists of 6 hexagonal compartments. Sedimentation unit of Sadu WTP is divided into two tanks. Each sedimentation tank is equipped with a tube settler with a slope of 60° and there are two sludge spaces per sedimentation tank. The filtration unit is designed using a rapid sand filter type with dual media filters, i.e. sand and anthrasite media and added gravel buffer.

Daftar Pustaka

- Alfatih, I.Z., Warnana, D.D., Wijaya, P.H. (2017). Klasifikasi Fasies pada Reservoir Menggunakan Crossplot Data Log P-Wave dan Data Log Density. Jurnal Teknik ITS, vol. 6 (1): B127-B130.
- Al-Kizwini, R.S.. (2015). Improvement of Sedimentation Process Using Inclined

Plates. *Mesopotamia Environmental Journal*, vol. 2, No.1: 100-114.

- Angreni, E. (2009). Review on Optimization of Conventional Drinking Water Treatment Plant. World Applied Sciences Journal 7(9): 1144-1151.
- Anupoju. S. (2016). What are Intake Structures?
 8 Types of Intake Structures. *The Constructor Civil Engineering Home.*
- Baharodi, A. (2016). Water Supply and Distribution Systems. *Essential of Oil and Gas Utilities*. Process Design, Equipment, and Operations, pages 225-328.
- Hasbiah, A.W., Rusmaya, D., Apriani, D. (2019). Sanitasi Berbasis Masyarakat di Pesantren Putri Al-Ittihad, Kabupaten Cianjur. Journal of Community Based Environmental Engineering and Management, vol. 3 (1): 1-8.
- Oana, T., Valentin, N., and Gabriel, L. (2011). Actual Stage of Water Filtration. *Journal of Engineering Studies and Research*, vol. 17, No.4.
- Smith, R., Van de Loo, J., Van den Boomen, M., Khakzad, N., Van Heck, G.J., and Wolfert, A.R.M. (2019). Long-term Availability Modelling of Water Treatment Plants. *Journal of Water Process* Engineering, vol. 28, 203-213.
- Qasim, S.R.. (2000). Wastewater Treatmnet Plant: Planning, Design, and Operation. New York.
- Setyowati, R.D.N., Junaidi, R. (2016). Analisis Routing Reservoir dalam Pengembangan Sumber Daya Air Kawasan Karst. *Al-Ard*, vol. 2 (1): 16-22.
- Tzoupanos, N.D. and Zouboulis, A.I. (2008). Coagulation-Flocculation Processes in Water/Wastewater Treatment: The Application of New Generation of Chemical Reagents. *International*

Conference on Heat Transfer, Thermal Engineering and Environment, pp 20-22.

Udimal, T.B, Jincai, Z., Ayamba, E.C., and Owusu. S.M. (2017). China's Water Situation; The Supply of Water and The Pattern of Its Usage. *International Journal of Sustainable Built Environment*, **6**, 491-500.